EXHIBIT B

GENERAL NOTES

1. [CONFORMANCE] ALL CONSTRUCTION SHALL CONFORM TO CURRENT GOVERNING CODES, AMENDMENTS, RULES, REGULATIONS, ORDINANCES, LAWS, ORDERS, APPROVALS, ETC THAT ARE REQUIRED BY APPLICABLE PUBLIC AUTHORITIES. IN THE EVENT OF CONFLICT, THE MOST STRINGENT REQUIREMENTS SHALL APPLY.
2. [CONDITIONS] THE GENERAL CONTRACTOR IS RESPONSIBLE FOR CHECKING CONTRACT DOCUMENTS, FIELD CONDITIONS, AND DIMENSIONS FOR ACCURACY AND CONFIRMING THAT THE WORK CAN BE BUILT OR DEMOLISHED AS SHOWN BEFORE PROCEEDING WITH WORK. IF THERE ARE QUESTIONS REGARDING THESE DRAWINGS OR OTHER COORDINATION QUESTIONS, THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING CLARIFICATION FROM THE EOR BEFORE PROCEEDING WITH THE WORK IN QUESTION OR RELATED TO WORK.
3. [OMISSIONS] ANY ERRORS, OMISSIONS, OR CONFLICTS FOUND IN VARIOUS PARTS OF THE CONSTRUCTION DOCUMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE EOR BEFORE PROCEEDING WITH THE WORK.
4. [COPYRIGHT] ALL IDEAS, DESIGNS, OR PLANS INDICATED OR REPRESENTED BY THIS DRAWING ARE OWNED BY AND ARE THE PROPERTY OF FRAMEWORK ENGINEERING - AND WERE CREATED, EVOLVED AND DEVELOPED FOR USE ON, AND IN CONNECTION WITH, THE SPECIFIED PROJECT. NONE OF THESE IDEAS, DESIGNS, OR PLANS SHALL BE USED BY ANY PERSON FOR ANY PURPOSE WITHOUT THE WRITTEN PERMISSION OF DUSTIN MUHN, PE OR JOHN VOEKEL, PE.
5. [DEMOLITION] PORTIONS OF STRUCTURE INDICATED AS DEMO ARE COORDINATED WITH ARCHITECTURAL DRAWINGS. WHERE FIELD CONDITIONS INDICATE DAMAGE, ROT, OR WEAR -- OR WHERE THE CONTRACTOR WOULD LIKE AN ALTERNATE CONSTRUCTION APPROACH THAT INCREASES THE SCOPE OF DEMOLITION, IT IS THEIR RESPONSIBILITY TO DOCUMENT ANY SUCH DAMAGE AND/OR CONDITION OF EXISTING CONDITIONS AS WELL AS CONTACTING CITY INSPECTORS TO VERIFY AND APPROVE REMOVAL OF ANY AND ALL MATERIALS.
6. [ROOM NAMES] ROOMS LABELED IN STRUCTURAL DRAWINGS DO NOT INDICATE LEGALITY OF UNITS, BATHROOMS, KITCHENS, OR LIVING SPACE. SEE ARCHITECTURAL DRAWINGS.
7. [SUPPLIERS] SUBSTITUTIONS OFFERED BY LUMBER YARD AND OTHER SUPPLIERS MUST BE VERIFIED BY EOR. NOT ALL PRODUCT TABLES CAPTURE THE DESIGN CRITERIA USED IN STRUCTURAL DRAWINGS, AND SUPPLIERS ARE USUALLY NOT LICENSED ENGINEERS. USING UNVERIFIED SUBSTITUTIONS MAY RESULT IN CONTRACTOR REMOVING INSTALLED PRODUCTS.

FOUNDATION NOTES

1. [DIMENSIONS] DO NOT SCALE DRAWINGS. VERIFY THAT STRUCTURAL DIMENSIONS CONFORM TO ARCHITECTURAL REQUIREMENTS. CONTACT EOR WHERE CONFLICT OCCURS.
2. [STRENGTH] SEE FOUNDATION PLAN FOR MINIMUM REQUIRED COMPRESSIVE CONCRETE STRENGTH (f'c). IF UN-DOCUMENTED, IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY THE EOR PRIOR TO POURING CONCRETE.
3. [MIX] CONCRETE SHALL BE HARD ROCK CONCRETE, USING PORTLAND CEMENT TYPE I OR II LOW ALKALINE AND SHALL ATTAIN ULTIMATE COMPRESSIVE STRENGTH WITHIN 28 DAYS. MAXIMUM CEMENT CONTENT = 6 SACKS/CU YD. MAXIMUM SLUMP = 4".
4. [WET TRENCHES] DO NOT ALLOW WATER TO STAND IN TRENCHES. IF BOTTOMS OF TRENCHES BECOME SOFTENED DUE TO RAIN OR OTHER WATER BEFORE CONCRETE IS CAST, EXCAVATE SOFTENED MATERIAL AND REPLACE WITH PROPERLY COMPACTED BACKFILL OR CONCRETE AT NO COST TO THE OWNER.
7. [INSPECTION] ALL EXCAVATION FORMS AND REINFORCING ARE TO BE INSPECTED BY THE LOCAL BUILDING INSPECTOR BEFORE PLACING CONCRETE.
8. [PLAIN CONCRETE] PLAIN CONCRETE (CONCRETE WITH MINIMAL OR NO REBAR) IS NOT PERMITTED. INSTALL BOLTS, ANCHORS, AND REINFORCING AND SECURELY TIE PRIOR TO PLACING CONCRETE.
9. [BATCHING] NO MORE THAN 90 MINUTES SHALL ELAPSE BETWEEN CONCRETE BATCHING AND CONCRETE PLACING.
10. [EMBEDDED PIPES] CONDUIT OR PIPES WITHIN CONCRETE SHALL NOT EXCEED 30% OF MEMBER THICKNESS, SHALL BE SPACED AT LEAST 4 DIAMETER APART, AND MAY NOT OCCUR WITHIN ONE MEMBER THICKNESS FROM EDGE.
11. [REBAR MATERIAL] ALL REINFORCING STEEL BAR SHALL CONFORM WITH THE STANDARD SPECIFICATIONS FOR DEFORMED BILLET STEEL FOR CONCRETE REINFORCEMENT. ASTM A615 AND ASTM A706. BARS #3,#4 SHALL BE GRADE 40 OR HIGHER. BARS #5 AND LARGER SHALL BE GRADE 60.
12. [REBAR CHAIRS] SUITABLE DEVICES (BRICKS, CHAIRS, STANDS, ANCHORMATES, DOBIES) SHALL BE USED TO HOLD REINFORCEMENT IN ITS TRUE HORIZONTAL AND VERTICAL POSITIONS. THESE DEVICES SHALL BE SUFFICIENTLY RIGID AND NUMEROUS TO PREVENT DISPLACEMENT OF THE REINFORCEMENT DURING THE PLACEMENT OF CONCRETE.
13. [ANCHOR BOLTS] ALL ANCHOR BOLTS SHALL BE A307 STEEL, $\frac{5}{8}$" DIAMETER, AND HAVE 7" MINIMUM EMBEDMENT. 3" x 3" x 0.229" WASHERS SHALL BE USED AT EACH LOCATION. ANCHOR BOLTS MAY BE SUBSTITUTED BY EPOXY ANCHORS OF EQUAL DIAMETER AND EMBEDMENT USING SIMPSON SET 3G EPOXY. SEE CONCRETE DETAILS FOR ANCHOR BOLT SUBSTITUTION OPTIONS & DETAILS. EXPANSION ANCHORS ARE NOT ACCEPTABLE.
14. [DIFFERENTIAL SETTLEMENT] FOOTING DESIGN EXECUTED CONSIDERING GOOD AND STABLE SOIL. FOR DIFFERENTIAL SETTLEMENT, CONSULT SOILS ENGINEER. FRAMEWORK ENGINEERING SHALL BE HELD HARMLESS AND INDEMNIFIED FOR ANY ARCHITECTURAL OR STRUCTURAL DAMAGES DUE TO DIFFERENTIAL FOUNDATION SETTLEMENT.
15. [EXISTING CONDITIONS] IF FIELD CONDITIONS DIFFER FROM SPECIFIED IN THIS PLAN, CONTRACTOR SHALL NOTIFY EOR TO CONSIDER STRUCTURAL CONSEQUENCES OR POTENTIAL REVISIONS.
16. [SHORING] CONTRACTOR RESPONSIBLE FOR SHORING DURING CONSTRUCTION AND/OR ENGAGEMENT OF A SHORING ENGINEER, WHERE REQUIRED. THE CONTRACTOR SHOULD BE RESPONSIBLE FOR ALL TEMPORARY EXCAVATIONS, SLOPES AND TRENCHES AT THE SITE AND DESIGN AND CONSTRUCTION OF ANY REQUIRED SHORING. SHORING AND BRACING SHOULD BE PROVIDED IN ACCORDANCE WITH ALL APPLICABLE LOCAL, STATE, AND FEDERAL SAFETY REGULATIONS, INCLUDING THE CURRENT OSHA EXCAVATION AND TRENCH SAFETY STANDARDS.
17. [SOILS] FOUNDATION SIZES, DEPTHS, AND REINFORCEMENT ARE AS RECOMMENDED WITH THE OWNER SOILS REPORT. GEOTECHNICAL ENGINEER TO PROVIDE FOUNDATION INSPECTIONS AND OBSERVATIONS AS OUTLINED IN LATEST SOILS REPORT. CONTRACTOR SHALL CONTACT GEOTECHNICAL ENGINEER WITH REASONABLE LEAD TIME TO ALLOW REVIEW OF FORMWORK, EXCAVATION, SUB-GRADES, AND PREPARED SOIL BEARING SURFACES PRIOR TO PLACEMENT OF FOUNDATION REINFORCING STEEL AND CONCRETE. ASSUMED VALUES SHALL BE FIELD VERIFIED BY THE THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.
18. [EPOXY] CONCRETE SHALL BE AGED TO A MINIMUM OF 21 DAYS BEFORE DRILLING AND INSTALLING EPOXY IS ALLOWED. EPOXY SHALL BE SIMPSON SET 3G UON

FRAMING NOTES

1. [DIMENSIONS] DO NOT SCALE DRAWINGS. VERIFY THAT STRUCTURAL DIMENSIONS CONFORM TO ARCHITECTURAL REQUIREMENTS. SHEARWALL LENGTHS NOTED ARE MINIMUM. CONTACT EOR WHERE CONFLICT OCCURS.
2. [FRAMING] ALL CONSTRUCTION SHALL COMPLY WITH THE STANDARDS OF THE QUALITY REQUIREMENTS OF THE NATIONAL DESIGN STANDARD (NDS) AND CODES LISTED UNDER 'APPLICABLE CODES', THIS SHEET.
3. [GRADES] ALL STUDS, PLATES SHALL BE DOUGLAS FIR #2 OR BETTER; ALL JOISTS, RAFTERS, POSTS, AND BEAMS SHALL BE DOUGLAS FIR SELECT STRUCTURAL; ALL FRAMING EXPOSED TO WEATHER OR TOUCHING CONCRETE SHALL BE PRESSURE TREATED DOUGLAS FIR, REDWOOD SELECT, IPE, CEDAR, MANUFACTURED DECKING, OR OTHER WEATHER-PROTECTED SPECIES. REFER TO ARCHITECTURAL OR LANDSCAPE DRAWINGS.
4. [MOISTURE CONTENT] ALL LUMBER SHALL HAVE A MOISTURE CONTENT OF 19% OR LESS PRIOR TO PLACEMENT.
5. [NAILING] ALL FASTENERS IN CONTACT WITH PRESSURE TREATED AND FIRE RETARDANT TREATED LUMBER, OR PERMANENTLY EXPOSED TO WEATHER SHALL BE HOT-DIPPED, ZINC-COATED, GALVANIZED, OR STAINLESS STEEL, IN ACCORDANCE WITH CODES LISTED UNDER 'APPLICABLE CODES', THIS SHEET.
6. [JOISTS] PLACE JOISTS MEMBERS WITH CROWN UP. DOUBLE ALL JOISTS UNDER PARALLEL PARTITIONS, UON. GLUE ALL JOISTS TO UNDERSIDE OF SHEATHING.
7. [CONNECTORS] ALL CONNECTORS AND HARDWARE NOT SPECIFIED SHALL BE SIMPSON STRONG-TIE, INC. WITHOUT EXCEPTIONS. IF PRODUCT CANNOT BE FOUND, CONSULT EOR OR LOCAL SIMPSON DISTRIBUTION REPRESENTATIVE.
8. [DRILLED HOLES] THE DIAMETER OF BORED HOLES FOR MACHINED BOLTS SHALL NOT BE LARGER THAN THE SPECIFIED BOLT SIZE PLUS $\frac{1}{32}$" AND SHALL USE WASHERS. IT IS RECOMMENDED THAT THE INSTALLER USE A DRILLING GUIDE FOR MEMBERS THICKER THAN 4" FOR STRAIGHTER, TRUER BORED HOLES.
9. [POSTS AND BEAMS] ALL BEAMS SHALL BEAR ON POSTS HAVING A WIDTH TO MATCH WIDTH OF BEAM. POSTS SUPPORTING MANUFACTURED LUMBER PRODUCTS (PSL, LVL, GLULAM) SHALL HAVE SIMPSON POST CAPS OR HANGERS AND POST BASES, UON.
10. [SILL PLATES] SILLS ON CONCRETE SHALL BE 3x PRESSURE TREATED DOUGLAS FIR. SILLS SHALL BE FASTENED TO THE CONCRETE WITH A MINIMUM OF TWO ANCHOR BOLTS PER PIECE SPACED NO MORE THAN 4' O.C. AND NO ANCHOR BOLTS LESS THAN 9" FROM THE END OF A PIECE.
11. [MANUFACTURED LUMBER] ENGINEERED LUMBER SHALL BE MANUFACTURED BY WEYERHAUSER, REDBUILT, OR AN APPROVED EQUIVALENT. ALL MANUFACTURER RESTRICTIONS APPLY.
12. [LVL STRENGTH] Fb = 2600 PSI, Fc = 750 PSI, Fv = 285 PSI, E = 1,900,000 PSI
13. [PSL STRENGTH] Fb = 2900 PSI, Fc = 750 PSI, Fv = 290 PSI, E = 2,000,000 PSI
14. [LSL STRENGTH] Fb = 1700 PSI, Fc = 680 PSI, Fv = 400 PSI, E = 1,550,000 PSI
15. [TJI STRENGTH] VARIES. SEE PLANS.
16. [WALLS] ALL WALLS ARE 2x STUDS @ 16" O.C. THICKNESS PER PLAN.

SHEATHING NOTES

1. [DIMENSIONS] DO NOT SCALE DRAWINGS. VERIFY THAT STRUCTURAL DIMENSIONS CONFORM TO ARCHITECTURAL REQUIREMENTS. SHEARWALL LENGTHS NOTED ARE MINIMUM. CONTACT EOR WHERE CONFLICT OCCURS.
2. ALL WOOD STRUCTURAL PANELS SHALL BE MARKED WITH APPROPRIATE TRADEMARK OF APA AND MEET ALL CORRESPONDING CRITERIA.
3. [DIRECTION HORIZ.] WOOD STRUCTURAL PANELS AT FLOORS AND ROOFS SHALL BE LAID WITH FACE GRAIN PERPENDICULAR TO JOISTS AND RAFTERS, UON.
4. [DIRECTION VERT.] WOOD STRUCTURAL PANELS AT WALLS SHALL BE LAID WITH LONG DIRECTION VERTICAL. BLOCK ALL EDGES. MINIMUM DIMENSION =24".
5. [FLOOR] UON, FLOOR SHEATHING SHALL BE T&G $\frac{23}{32}$" THICK WITH SPAN RATING 48/20, EXPOSURE I WITH 10D NAILS @ 12" O.C FIELD NAILING, @ 6" O.C. EDGE NAILING, AND @ 3" O.C. BOUNDARY NAILING. CONTRACTOR MAY OMIT T&G WHERE EDGES ARE BLOCKED.
6. [ROOF] UON, ROOF SHEATHING SHALL BE $\frac{23}{32}$" THICK WITH SPAN RATING 32/16, EXPOSURE I OR 5-PLY T&G WITH 10D NAILS @ 12" O.C FIELD NAILING, @ 6" O.C. EDGE NAILING, AND @ 3" O.C. BOUNDARY NAILING. PROVIDE PLY CLIPS BETWEEN JOINTS WHERE PANELS ARE NOT BLOCKED.
7. [GAP] ALL SHEATHING PANELS SHALL BE INSTALLED SUCH THAT THERE IS AN $\frac{1}{8}$" GAP BETWEEN PANEL EDGES TO ALLOW FOR SWELLING AND/OR EXPANSION.

SPECIAL INSPECTION & STRUCTURAL OBSERVATION NOTES

1. [REQUIRED] PURSUANT OF OSSC 2022 BUILDING CODE, SECTIONS 1704, 1707, AND 1708, SPECIAL INSPECTIONS ARE REQUIRED TO BE PERFORMED BY A THIRD PARTY WITNESSING AGENCY.
2. [RESPONSIBILITY] CONTRACTOR IS RESPONSIBLE FOR SCHEDULING AND COORDINATING INSPECTIONS AND OBSERVATIONS WITH APPROPRIATE NOTICE AND FOR ENSURING THAT THE WORK IS SATISFACTORY TO BE APPROVED.
3. [DISCIPLINES] INSPECTIONS & OBSERVATIONS LISTED ON THIS DRAWING SET ARE RELATED TO STRUCTURAL FEATURES OF THE PROJECT. THE WORK OF OTHER DISCIPLINES MAY REQUIRE TESTING AND INSPECTION THAT IS ADDITIONAL AND NOT LISTED ON STRUCTURAL SHEETS.

RECOMMENDED STRUCTURAL OBSERVATIONS

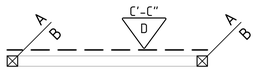
LIST OF REQUIRED STRUCTURAL OBSERVATIONS TO BE PERFORMED BY FRAMEWORK ENGINEERING. THIS REVIEW SHALL NOT BE CONSTRUED AS SPECIAL INSPECTION. ALLOW 3 BUSINESS DAYS NOTICE TO EOR.

1. [FOUNDATION] REBAR PLACEMENT, ANCHOR BOLT PLACEMENT, AND CAST-IN ANCHORAGE PLACEMENT PRIOR TO POURING CONCRETE; FORMWORK DIMENSIONS.
2. [FRAMING CONNECTIONS] POSTS, BEAMS, AND POST/BEAM CONNECTIONS; PRIOR TO CONCEALMENT BY DRYWALL OR INTERIOR FINISHES.
3. [LATERAL CONNECTIONS] HOLDOWNS, COLLECTORS, STRAPS, TIES AND DRAG STRUTS
4. [SHEAR NAILING] NAIL SPACING, NAIL HEAD PENETRATION, DISCONTINUITIES

ABBREVIATIONS

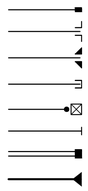
AB	ANCHOR BOLT	ICC	INTERNATIONAL CODE COUNCIL
ALT	ALTERNATE	INT	INTERIOR
ARCH	ARCHITECTURAL	KSI	KIPS PER SQUARE INCH
ASD	ALLOWABLE STRESS DESIGN	LBS	POUNDS
AWC	AMERICAN WOOD COUNCIL	LDGR	LEDGER
B&S	BLOCK & STRAP	LL	LIVE LOAD
BLK'G	BLOCKING	LSL	LAMINATED STRAND LUMBER
BRG	BEARING	LVL	LAMINATED VENEER LUMBER
BTWN	BETWEEN	MECH	MECHANICAL
CALC	CALCULATIONS	MISC	MISCELLANEOUS
CANT	CANTILEVER	NTS	NOT TO SCALE
CIP	CAST IN PLACE	OC	ON CENTER
CJ	CONTROL JOINT	O-O	OUT TO OUT
CMU	CONCRETE MASONRY UNIT	OSB	ORIENTED STRAND BOARD
COL	COLUMN	PFA	POST FROM ABOVE
COLL	COLLECTOR	PLY	PLYWOOD
CON	CONCRETE	PSI	POUNDS PER SQUARE INCH
CONT	CONTINUOUS	PSL	PARALLEL STRAND LUMBER
CP	COMPLETE PENETRATION	PT	PRESSURE TREATED
DBL	DOUBLE	REBAR	REINFORCEMENT BAR
DEMO	DEMOLITION	SAD	SEE ARCHITECTURAL DRAWING
DF	DOUGLAS FIR	SDS	STRONG-DRIVE WOOD SCREW
DF#1	DOUGLAS FIR GRADE 1	SHTG	SHEATHING
DF#2	DOUGLAS FIR GRADE 2	SMF	SPECIAL MOMENT FRAME
DIA	DIAMETER	SOG	SLAB ON GRADE
DIAG	DIAGONAL	SPEC	SPECIFIED
DL	DEAD LOAD	SS	STAINLESS STEEL
EN	EDGE NAILING	SST	SIMPSON STRONG-TIE
EOR	ENGINEER OF RECORD	SSW	STEEL STRONG-WALL (SIMPSON)
EQ	EQUAL	STAG'D	STAGGERED
EXP	EXPANSION	STD	STANDARD
EXT	EXTERIOR	SWS	SHEAR WALL SCHEDULE
FN	FIELD NAILING	SYM	SYMMETRIC
FOUN	FOUNDATION	T&B	TOP AND BOTTOM
FT	FOOT	T&G	TONGUE AND GROOVE
GA	GAUGE	TJI	TRUSS JOIST I-JOIST
GALV	GALVANIZED	TN	TOE NAIL
GEO	GEOTECHNICAL	TP	TOP PLATE
GYP	GYPSUM BOARD	TYP	TYPICAL
HDR	HEADER	UON	UNLESS OTHERWISE NOTED
HGR	HANGER	VIF	VERIFIED IN FIELD
HT	HEIGHT	W/	WITH
HVAC	HEATING VENT & AIR COND.	WSW	WOOD STRONG-WALL

SYMBOLS LEGEND



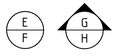
SHEAR WALL TAG
A = POST SIZE & HARDWARE AT TOP OF POST
B = HARDWARE AT BOTTOM OF POST
C = SHEARWALL WIDTH, MEASURED TO OUTSIDE OF POSTS ("P" INDICATES THAT SHEARWALL IS PERFORATED)
D = SHEARWALL TYPE (SEE SWS)

SPANNING MEMBER TAGS



STRONG-WALL TAG

DETAIL TAG / ELEVATION TAG
E = SECTIONAL DETAIL NUMBER
F = DETAIL SHEET NUMBER
G = ELEVATIONAL DETAIL NUMBER
H = DETAIL SHEET NUMBER



POST & ANCHOR TAG
I = ANCHORAGE TYPE (SEE PLAN FOR SCHEDULE)
J = POST SIZE
K = HARDWARE AT TOP OF POST
L = HARDWARE AT BOTTOM OF POST

SHADE CANOPY

22055 S BEAVERCREEK RD
BEAVERCREEK, OR 97004
APN C121433

PROJECT PARTICIPANTS

STRUCTURAL ENGINEER	OWNER(S)	LANDSCAPE ARCHITECT
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FRAMEWORKENG.COM		
503 345-3075		

SCOPE OF WORK

FREE STANDING PERGOLA NEAR AN EXISTING RETAINING WALL. DESIGN INTENT RECOMMENDS A WOODFRAME STRUCTURE WITH COLUMN PAIRS AT THE CORNERS. STRUCTURAL SCOPE INCLUDES:

1 / DESIGN OF WOODFRAME PERGOLA FOR BOTH VERTICAL AND LATERAL LOADS
2 / DETAILING OF ROOF-TO-BEAM-TO COLUMN CONNECTION USING CONCEALED FASTENERS
3 / DETAILING OF POST-BASE CONNECTION TO RESIST MOMENT USING CONCEALED HARDWARE
4 / DESIGN OF FOUNDATION ELEMENTS TO RESIST COLUMN LOADS AND AVOID SURCHARGING THE EXISTING RETAINING WALL

DRAWING INDEX

S000 GENERAL NOTES
S100 PLANS
S500 DETAILS

APPLICABLE CODES

2016 - ASCE 7 MINIMUM BUILDING LOADS
2022 - OREGON STRUCTURAL SPECIALTY CODE
2018 - NATIONAL DESIGN STANDARD / SDPWS
APPLICABLE [CITY BUILDING CODES]

DESIGN PARAMETERS

LOAD ASSUMPTIONS	
ROOF LOAD (DL, LL)	15 PSF, 20 PSF
FLOOR LOAD (DL, LL)	15 PSF, 40 PSF
DECK LOAD (DL, LL)	15 PSF, 60 PSF
SNOW LOAD (LL)	25 PSF
RAIN LOAD (LL)	5 PSF
GUARDRAILS	50PLF OR 200 LB
SITE AND SOIL	
RISK / OCCUPANCY CATEGORY	II
SOIL TYPE	D
SEISMIC DESIGN CATEGORY	D
SEISMIC PARAMETERS	
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE
SEISMIC RESISTANT SYSTEM	TIMBER CANT. COLUMNS
DIAPHRAGM FLEXIBILITY	FLEXIBLE
SITE LATITUDE	45.2867508 °N
SITE LONGITUDE	122.5298951 °W
SDS	0.601 g
SD1	NULL g
S1	0.345
DESIGN COEFFICIENTS, R	1
OVERSTRENGTH FACTOR, W	1.25
DEFLECTION AMPLIFICATION FACTOR, CD	1
REDUNDANCY FACTOR, P	1
IMPORTANCE FACTOR, IE	1
CS	0.601
ALLOWABLE STORY DRIFT	0.025
WIND PARAMETERS	
ENCLOSURE CLASSIFICATION	OPEN
BASIC WIND SPEED (MPH)	110
WIND DIRECTIONALITY FACTOR	0.85
EXPOSURE CATEGORY	C
TOPOGRAPHIC FACTOR	1
GUST EFFECT FACTOR	DOES NOT APPLY
SURFACE ROUGHNESS CATEGORY	B
SOIL STRENGTH	
ALLOWABLE SOIL BEARING PRESSURE	1500 PSF
PASSIVE SOIL PRESSURE	350 PCF
ACTIVE SOIL PRESSURE	40 PCF
COEFFICIENT OF FRICTION	-
COHESION	-
SKIN FRICTION	-

VICINITY MAP

AERIAL VIEW

FRAMEWORK ENGINEERING
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104 VICKSBURG ST
SAN FRANCISCO, CA 94114
415 604-3876
107 SE WASHINGTON ST
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503 345-3075

REGISTERED PROFESSIONAL ENGINEER
90078
OREGON
JOHN VOEKEL
RENEWS: JUNE 20, 2026

SHADE CANOPY
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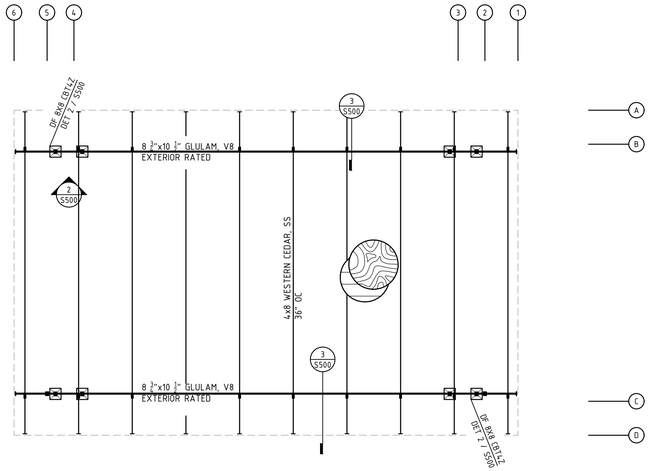
JOB NUMBER	R24-037
PREPARED BY	JV
REVIEWED BY	DM
PERMIT SET	16 JUL 2024

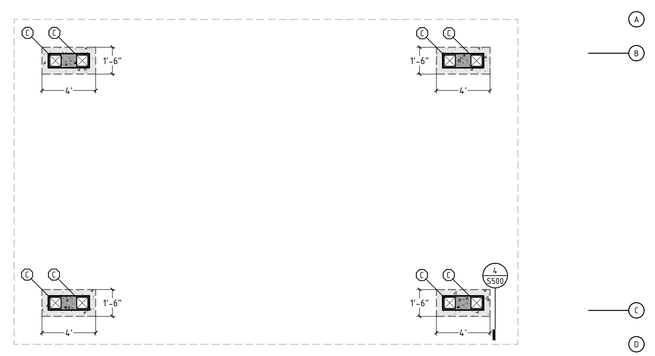
GENERAL NOTES

S000



1 ROOF FRAMING

PLAN 1/4" = 1'



2 FOUNDATION & ANCHORAGE

PLAN 1/4" = 1'

DIAPHRAGM SCHEDULE

7/16" STRUCT I PLY OVER 2x6 T&G DECKING
USE 10d COMMON NAILS:
3" BOUNDARY NAILING
6" EDGE NAILING
12" FIELD NAILING

3 S500

CONCRETE LEGEND, f'c = 3000 psi

NEW FOUNDATION CURB / STEM OCCURS ABOVE GRADE

NEW FOUNDATION FOOTING OCCURS BELOW GRADE

ANCHORAGE SCHEDULE

DETAIL

C CUSTOM POST BASE — 1 S500

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REGISTERED PROFESSIONAL ENGINEER
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JOHN VOSECKY
RENEWS: JUNE 20, 2026

SHADE CANOPY

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APN C121433

JOB NUMBER	R24-037
PREPARED BY	JV
REVIEWED BY	DM
PERMIT SET	16 JUL 2024

PLANS

S100

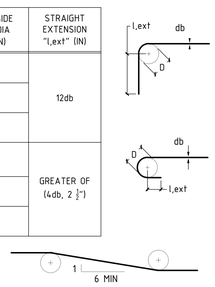
CONCRETE EXPOSURE	MEMBER	REINFORCEMENT SIZE	SPECIFIED COVER, in.
CONTACT WITH GROUND	ALL	ALL	3 in.
EXPOSED TO WEATHER	ALL	#6 - #18	2 in.
		#3 - #5	1 ½ in.
INTERIOR CONDITION	SLABS, JOISTS, WALLS	#14 - #18	1 ½ in.
		#3 - #11	¾ in.
	BEAMS, COLUMNS, PEDESTALS, TENSION TIES	ALL	1 ½ in.

SPECIFIED COVER REFERS TO THE DISTANCE BETWEEN FACE OF CONCRETE AND OUTSIDE OF BAR DIAMETER

A CLEAR COVER FOR CONCRETE REINFORCEMENT

ACI 318-14 TABLE 20.6.1.3.1

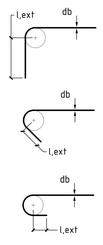
BEND	BAR SIZE "db"	MIN. INSIDE BEND DIA "D" (IN)	STRAIGHT EXTENSION "l,ext" (IN)
90-DEGREE HOOK	#3 - #8	6db	12db
	#9 - #11	8db	
	#14 - #18	10db	
180-DEGREE HOOK	#3 - #8	6db	GREATER OF (4db, 2 ½")
	#9 - #11	8db	
	#14 - #18	10db	



B STANDARD REINFORCEMENT BENDS - LONGITUDINAL

ACI 318-14 TABLE 25.3.1

BEND	BAR SIZE "db"	MIN. INSIDE BEND DIA "D" (IN)	STRAIGHT EXTENSION "l,ext" (IN)
90-DEGREE HOOK	#3 - #5	4db	GREATER OF (6db, 3")
	#6 - #8	6db	12db
135-DEGREE HOOK	#3 - #5	4db	GREATER OF (6db, 3")
	#6 - #8	6db	
180-DEGREE HOOK	#3 - #5	4db	GREATER OF (4db, 2 ½")
	#6 - #8	6db	



NOTES:

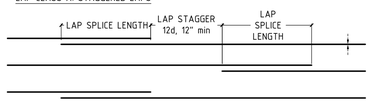
1. ALL BENDS SHALL BE MADE COLD.
2. #14 AND #18 BARS SHALL BE BEND TESTED AND LAB APPROVED.
3. DO NOT BEND BARS ALREADY CAST IN CONCRETE.
4. 135-DEGREE HOOKS NOT PERMITTED FOR LONG. BARS.

C STANDARD REINFORCEMENT BENDS - STIRRUPS & TIES

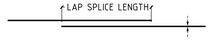
ACI 318-14 TABLE 25.3.2

WALL AND FOOTING LAP SPLICE SCHEDULE: GRADE 60											
CONCRETE STRENGTH f'c	LAP CLASS	REBAR SIZE "d"									
		#3	#4	#5	#6	#7	#8	#9	#10	#11	#14
		LAP SPLICE LENGTH, INCHES									
3000	A	12	15	22	27	33	48	55	62	70	77
	B	15	19	29	36	43	62	74	80	90	100

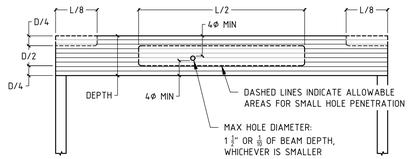
LAP CLASS A: STAGGERED LAPS



LAP CLASS B: INDIVIDUAL SPLICES



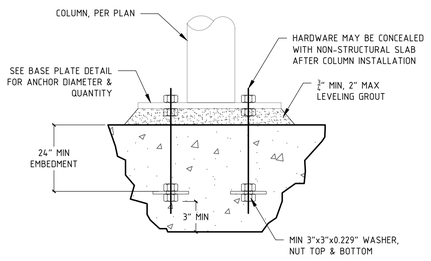
D STANDARD REINFORCEMENT LAP SPLICES



NOTE:
SMALL HOLES ARE PERMITTED FOR UNIFORMLY LOADED, SIMPLY SUPPORTED GLULAMS ONLY.
[2014 APA FIELD NOTCHING AND DRILLING OF GLUED LAMINATED TIMBER BEAMS, FORM NO S560H]

E ALLOWABLE GLULAM HOLES

ELEVATION NO SCALE



F TYPICAL CAST-IN HEADED ANCHOR COLUMN BASE

SECTION 2" = 1'

STEEL NOTES:

1. [DIMENSIONS] DO NOT SCALE DRAWINGS. VERIFY THAT STRUCTURAL DIMENSIONS CONFORM TO ARCHITECTURAL REQUIREMENTS. CONTACT EOR WHERE CONFLICT OCCURS.
2. [INSPECTION] SPECIAL INSPECTION IS REQUIRED AT SEISMIC CONNECTIONS. NDT AND ULTRASONIC TESTING SHALL BE CONDUCTED BY PERSONNEL QUALIFIED IN ACCORDANCE WITH AWSD1.8 STANDARDS.
3. [MATERIAL] ALL SHAPES SHALL MEET ASTM & AISC STANDARDS. USE THE FOLLOWING MATERIAL GRADES BY SECTION TYPE:

W-SECTIONS	ASTM A992	Fy = 50 KSI, Fu = 65 KSI
HSS SECTIONS, □	ASTM A500	Fy = 46 KSI, Fu = 58 KSI
HSS SECTIONS, ○	ASTM A500	Fy = 42 KSI, Fu = 58 KSI
L-ANGLES	ASTM A36	Fy = 36 KSI, Fu = 58 KSI
PLATES & BARS	ASTM A36	Fy = 32 KSI, Fu = 58 KSI

4. [RESPONSIBLE] CONTRACTOR IS RESPONSIBLE FOR THE FULL COMPLIANCE OF ABOVE SPECIFICATIONS WHICH INCLUDE, BUT NOT LIMITED TO, OVERSIZED HOLES, HARDENED WASHERS, SURFACE TREATMENT, FASTENER TENSION, INSPECTION, ETC.
5. [WELDING] ALL WELDING SHALL BE PERFORMED IN ACCORDANCE WITH ALL THE APPLICABLE PROVISIONS OF THE AWS D1.1M BY THE AMERICAN WELDING SOCIETY EXCEPT AS MODIFIED BY AISC 360 SPECIFICATION SECTION J2 AND APPLICABLE BUILDING CODE
6. [ELECTRODES] WELDING ELECTRODES SHALL BE E70XX FOR SHIELD METAL ARC WELDING. QUALIFIED WELDER SHALL BE CERTIFIED PER AWS D1.1 STANDARDS, WHICH INCLUDE THE TYPE OF WELDING, POSITIONS, DATE QUALIFIED, AND FIRM/INDIVIDUAL CERTIFYING THE QUALIFICATION TESTS.
7. [FASTENERS] ALL BOLTED & FASTENED ASSEMBLIES SHALL MEET ASTM & AISC STANDARDS.
   7.1. UON, BOLTED JOINTS SHALL BE SNUG-TIGHT ATTAINED WITH A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF AN IRONWORKER USING AN ORDINARY SPUD WRENCH TO BRING THE CONNECTED PLIES INTO FIRM CONTACT.
   7.2. BOLT HOLES SHALL BE FABRICATED ⅛" LARGER THAN BOLT / ROD DIAMETER.
   7.3. UON, USE THE FOLLOWING MATERIAL GRADES BY SECTION TYPE:

COMMON BOLTS	ASTM A325	Fu = 120 KSI
NUTS	ASTM A563	
WASHERS	ASTM F436	
ANCHOR RODS	ASTM F1554, WELD.	Fy = 55 KSI,

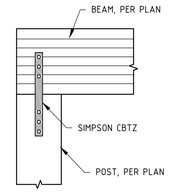
8. [HIGH-STRENGTH BOLTS] RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS, SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 or A490 BOLTS. AISC DESIGN GUIDE 17
   8.1. HIGH-STRENGTH BOLTS SHALL BE PROTECTED FROM DIRT & MOISTURE IN CLOSED CONTAINERS AT THE SITE OF INSTALLATION, SHALL NOT BE INCORPORATED INTO THE WORK WHERE RUST OR DIRT HAS ACCUMULATED, SHALL INCLUDE DOCUMENTATION FROM THE MANUFACTURER TO ENSURE COMPONENTS ARE IDENTIFIABLE AND MEET THE REQUIREMENTS OF THE APPLICABLE ASTM SPECIFICATION, AND SHALL NOT BE RE-USED (RCSC SECTION 2)
   8.2. ENGINEER SHALL SPECIFY BOLT ASTM DESIGNATION, BOLT TYPE 1 OR 3, BOLT FINISH, JOINT TYPE, NUT GRADE & FINISH, WASHER TYPE & FINISH. WHERE SPECIFICATION HAS NOT BEEN PROVIDED, CONTACT EOR FOR INFORMATION (RCSC TABLE 2.1). HIGH-STRENGTH BOLTS SHALL COMPLY WITH THE FOLLOWING GRADES & STANDARDS:

ASTM A325	Fu = 120 KSI
ASTM A490	Fu = 150 KSI
ASTM F1852	Fu = 120 KSI

9. [SLIP-CRITICAL JOINTS] ENGINEER SHALL SPECIFY FAYING SURFACES OF SLIP-CRITICAL JOINTS (RCSC SECTION 3.2.2, TABLE 4.1), SHALL SPECIFY WASHER REQUIREMENTS AND OVERSIZED HOLE REQUIREMENTS (RCSC TABLE 6.1), SHALL SPECIFY INSTALLATION TENSION TO BE VERIFIED WITH A HYDRAULIC TENSION CALIBRATOR (RCSC SECTION 7, AISC SPECIFICATION TABLE J3.1), SHALL SPECIFY INSTALLATION INSTRUCTIONS (RCSC SECTION 8), AND SHALL SPECIFY INSPECTION REQUIREMENTS OF CONNECTIONS (RCSC SECTION 9). HOLLOW SECTIONS NOT ALLOWED FOR SLIP-CRITICAL OR PRE-TENSION BOLTED CONNECTIONS.

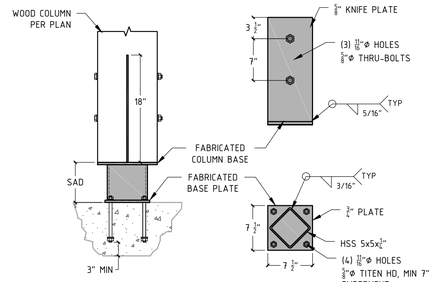
CBTZ INSTALLATION INSTRUCTIONS
SEE SIMPSON STRONG-TIE LITERATURE (T-C-CBTZINS)

1. [DRILL] USE AN 1 ½" AUGER BIT TO DRILL A VERTICAL HOLE INTO THE END GRAIN OF THE POST, CENTER OF CROSS SECTION [5 ½" DEEP FOR CBT2Z, 7" FOR CBT4Z]
2. [POST PINS] USE THE PROVIDED TEMPLATE TO LAYOUT THE PIN LOCATIONS IN THE POST. USE A ½" AUGER BIT AND DRILL STRAIGHT [RECOMMENDED USE SCRAP BLOCK ON BACKSIDE OF POST TO MINIMIZE WOOD SPLINTERING]
3. [BEAM] REPEAT STEPS 1 & 2 FOR THE BEAM [4 ½" DEEP FOR CBT2Z, 7" FOR CBT4Z]



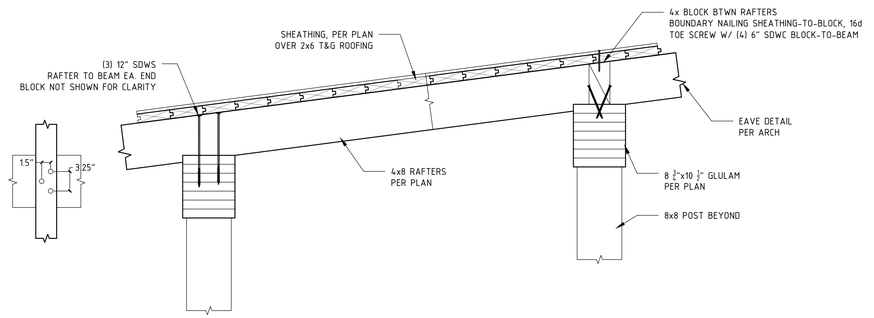
2 CONCEALED BEAM TIE [CBTZ]

SECTION 1" = 1'



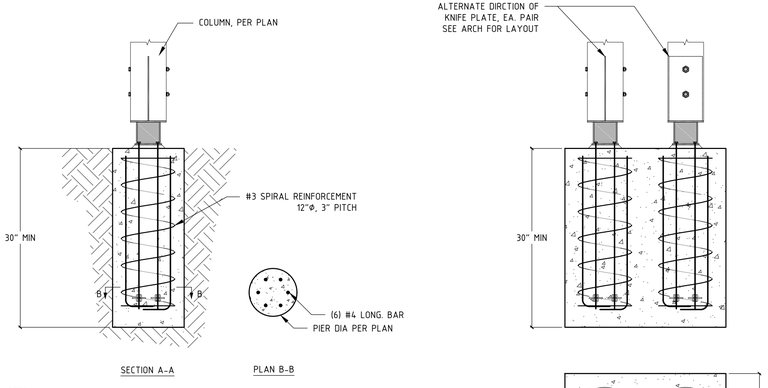
1 FABRICATED COLUMN BASE

SECTION 1" = 1'



3 RAFTER & BEAM FRAMING

SECTION 1" = 1'



SECTION A-A PLAN B-B

NOTES:

1. IF NON-COMPACTED FILL IS PRESENT AT THE FOOTING LOCATION, OVER-EXCAVATE TO NATIVE SOIL AND BACKFILL IN 6" LIFTS.

   FOR ANY OVER-EXCAVATION COMPLETED IN THE AREA OF FOOTINGS OR SLABS, THE BACKFILL MATERIAL SHALL CONSIST OF FREE-DRAINING, WELL-GRADED, CRUSHED AGGREGATE BASE WITH A MAXIMUM PARTICLE SIZE OF ¾ INCH. THE ROCK SHALL NOT CONTAIN MORE THAN 5% FINES (MATERIAL PASSING THE NO. 200 SIEVE, AS TESTED BY ASTM D-1140). THE ROCK SHALL BE COMPACTED TO A DRY DENSITY OF AT LEAST 92% OF ITS MOD. COMPACTION TESTS ARE REQUIRED FOR BACKFILLS OVER TWO (2) FEET AND EVERY TWO (2) FEET

4 ISOLATED FOOTING DETAIL

SECTION & PLAN 3/4"" = 1'

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415 604-3876
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REGISTERED PROFESSIONAL ENGINEER
90078
OREGON
JOHN VOSKUIL
RENEWS: JUNE 20, 2026

SHADE CANOPY

22055 S BEAVERCREEK RD
BEAVERCREEK, OR 97004
APN C121433

JOB NUMBER	R24-037
PREPARED BY	JV
REVIEWED BY	DM
PERMIT SET	16 JUL 2024

DETAILS

S500